

King Abdulaziz University Engineering College Department of Production and Mechanical System Design



MENG 270 Mechanics of Materials

Final Exam Wednesday: 22/11/1424 H Time Allowed: Two Hours

Name:	Sec. No.:	ID No.:
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Question 1	10
Question 2	10
Question 3	10
Question 4	10
Question 5	10
Question 6	10
TOTAL	60

Instructions

- 1. There are totally 6 problems in this exam.
- 2. Show all work for partial credit.
- 3. Assemble your work for each problem in logical order.
- 4. Justify your conclusion. I cannot read minds.

Closed-book Exam Time one hour Wednesday: 22/11/1424 H

Student Name:			Sec. No.:		ID No.:
Q1:	/10	Q2:	/10	Q3:	/10

1. Are these statements **true** or **false**?

 a. The shear strain in a rod is the deformation per unit length b. The Hook's law states that for small deformations the stress is directly proportional to the strain. c. The ratio of the lateral strain over the axial strain is called <i>Poisson's ratio</i>. d. The hoop stress of spherical thin-walled pressure vessels is twice as large as the longitudinal stress. e. Ductile materials are characterized by the fact that, when subjected to a tensile test, they fail suddenly through fracture without any prior yielding. f. The failure criterion most frequently used for brittle materials is the maximum-shear-stress criterion. g. The plane of maximum shearing stress is at 45 degree to the principal plane. h. The beam which has a fixed end at <i>x</i>=0 and is supported by a roller at <i>x</i>=<i>L</i> is statically indeterminate. i. If the stress on a transverse section of a column is less than the allowable strength then you can conclude that the column has been properly designed. j. The strain energy density is equal to the area under the load-deformation diagram. 			True	False
 b. The Hook's law states that for small deformations the stress is directly proportional to the strain. c. The ratio of the lateral strain over the axial strain is called <i>Poisson's ratio</i>. d. The hoop stress of spherical thin-walled pressure vessels is twice as large as the longitudinal stress. e. Ductile materials are characterized by the fact that, when subjected to a tensile test, they fail suddenly through fracture without any prior yielding. f. The failure criterion most frequently used for brittle materials is the maximum-shear-stress criterion. g. The plane of maximum shearing stress is at 45 degree to the principal plane. h. The beam which has a fixed end at x=0 and is supported by a roller at x=L is statically indeterminate. i. If the stress on a transverse section of a column is less than the allowable strength then you can conclude that the column has been properly designed. j. The strain energy density is equal to the area under the load-deformation diagram. 	a	The shear strain in a rod is the deformation per unit length		
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2. A beam is constructed by gluing three long, rectangular x-cross section piece of wood so that the resulting x-section is as shown below in Figure (1a). The loading is such that, at a particular transverse section, the internal shear force and the bending moment are as shown in Figure (1b).



Use the point labeled on Figure (1a) to complete the statement below. (There may be more than one answer for each statement):

a) Zero normal stress occurs at Point(s)	С		
b) Zero shear stress occurs at Point(s)	Е, А	<u> </u> .	
c) Maximum compressive stress occurs at Point(s)		Е	
d) Maximum shear stress occurs at Point(s)		С	_
e) Maximum tensile stress occurs at Point(s)	А		

- 3. The beam ABCD is loaded by a force W=30 KN by the arrangement shown in the Figure (2). The cable passes over a small frictionless pulley at B and is attached at E to the vertical arm. Calculate the following:
 - a) The reaction forces at *A* and *D*.
 - b) The axial forces N at section C, which is just to the left of the vertical arm.
 - c) The shear force V at section C.
 - d) The bending moment *M* at section *C*.



Solution

$$\sum F_x = 0: \qquad A_x = 24 \text{ KN}$$

$$\sum F_y = 0: \qquad A_y + D_y = 48 \text{ KN}$$

$$\sum M_A = 0: \qquad -30(2) - 18(4) + 36 + D_y(6) = 0$$

$$D_y = 16 \text{ KN} \text{ and } A_y = 32 \text{ KN}$$



Answers:

- a) $A_x = 24 \text{ KN}$, $A_y = 20 \text{ KN}$, and $D_y = 28 \text{ KN}$.
- b) *N*=24 KN.
- c) V=2 KN.
- d) M=68 KN.

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Q6:

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4. Each member of the truss shown in Figure (3) is made of aluminum (E=72 GPa). If the cross section area of the member BC is 2000 mm² and of the member CD is 2500 mm². Determine the strain energy of the truss.

Q5:



Figure (3)

Solution

Q4:

Strain Energy of the truss is given by

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$$\Sigma \frac{F_i^2 L_i}{2A_i E}$$

Where, F_i is the force in a given member under the combined loading. **Forces in members** Consider the equilibrium of joint C as shown in Figure (a) $F_{BC(x)} = F_{BC} \cos 36.87 = 0.8 F_{BC}$ $F_{BC(y)} = F_{BC} \sin 36.87 = 0.6 F_{BC}$ 80 KN $F_{DC(x)} = F_{DC} \sin 22.6 = 0.385 F_{DC}$ F_{BC(x)} 30 KN $F_{DC(y)} = F_{DC} \cos 22.62 = 0.923 F_{DC}$ 31.5 $\Sigma F_x = 0$ 2.26° $F_{DC(x)} - F_{BC(x)} - 30 = 0$ Þ F_{BC(y)} F⊮ $0.385 F_{DC} - 0.8 F_{BC} = 30$ (1)F_{DC(x)} Грс $\Sigma F_{\rm y} = 0$ Figure (a) $F_{DC(y)} - F_{BC(y)} - 80 = 0$ $0.923 \ F_{DC} - 0.6 \ F_{BC} = 80$ (2) Solving equations (1) & (2)

$$F_{BC} = 6.127$$
 kN
 $F_{DC} = 90.65$ kN

Length of members

$$L_{BC} = \sqrt{2.4^2 + 3.2^2} = 4 m$$
$$L_{DC} = \sqrt{2.4^2 + 1^2} = 2.6 m$$

 $\begin{array}{l} \textbf{Strain Energy} \\ U = U_{BC} \ + U_{DC} \end{array}$

$$U = \frac{F_{BC}^2 L_{BC}}{2 A_{BC} E} + \frac{F_{DC}^2 L_{DC}}{2 A_{DC} E}$$

$$U = \frac{\left(6.127 \times 10^{3}\right)^{2} \times 4}{2 \times 2000 \times 10^{-6} \times 72 \times 10^{9}} + \frac{\left(90.65 \times 10^{3}\right)^{2} \times 2.6}{2 \times 2500 \times 10^{-6} \times 72 \times 10^{9}}$$

- 5. Beam *CE* rests on beam AB as shown in Figure (4). Knowing that a W 250 x 49.1 rolled steel shape is used for each beam (E = 200 GPa), determine the following:
 - a) The deflection at point D due to point load on beam CE.
 - b) Forces acting on beam AB.
 - c) Deflection at point C on beam AB.
 - d) Total deflection at point D.



Figure (4)

Solution

- (a) For convenience let us split the given structure into two parts as shown in figure A little consideration will show that the deflection y_D at point D will be equal to
- y_D = Deflection of beam CD due to load 130 kN at D+ Deflection of beam AB due

combined loading at point C.

Deflection of beam CD

Let us consider simply supported beam CD, with point load

130 kN at point D.

a=1.2 m; \dot{b} = 1.2 m; L = 2.4 m; x= L/2; I = 70.8 x 10⁻⁶ m⁴ (from table) 130 kN Using relation, $y_1 = \frac{PL^3}{48 E I}$ 65kN 65 kN $y_1 = \frac{130 \times 1000 \times 2.4^3}{48 \times 200 \times 10^9 \times 70.8 \times 10^{-6}} = 0.002644 \, m$ 65kN 65kN $\rightarrow | \leftarrow 1.2 \rightarrow | 0.6m | \leftarrow$ $y_1 = 2.644 \text{ mm}$ \rightarrow 0.6m \leftarrow 1.2m \rightarrow Deflection of beam AB at point C Figure (a) Now we consider simply supported beam AB with two point loads; 65 kN each acting at point C and D. The deflection y₂ at point C is determined by using superposition theorem. Deflection due to point load at point C

a=0.6 m; b=3.0 m; L=3.6 m; x=a

Using relation

$$y' = \frac{Pa^{2}b^{2}}{3E1L}$$

$$y' = \frac{65 \times 100 \times 0.6^{2} \times 3^{2}}{3 \times 200 \times 10^{9} \times 70.8 \times 10^{-6} \times 3.6} = 0.001377 \ m$$

$$y' = 1.377 \ mm$$
Deflection at C due to point load at D
a= 3.0 m; b = 0.6 m, L = 3.6 m; x < a
Using relation

$$y'' = \frac{Pb}{6EIL} \left[x^{3} - (L^{2} - b^{2})x \right]$$

$$y'' = \frac{65 \times 1000 \times 0.6}{6 \times 200 \times 10^{9} \times 70.8 \times 10^{-6} \times 3.6} \left[0.6^{3} - (3.6^{2} - 0.6^{2}) \times 0.6 \right] = 0.000936 \ m$$

$$\therefore y_{2} = y' + y'' = 1.377 + 0.936$$

$$y_{2} = 2.313$$

$$y_{D} = y_{1} + y_{2} = 2.644 + 2.313$$

$$y_{D} = 4.96 \ mm$$

6. Knowing that the couple shown in Figure (5) acts in vertical plane, determine the stress at point A and at point B.



Figure (5)

Solution

Considering that the cross sectional area the centroidal moment of inertia

of the section is; $I = I_{(rectangle)} - 2 I_{(circle)}$ $I = \frac{bh^3}{12} - 2\left(\frac{\pi d^4}{64}\right)$ $I = \frac{120 \times 60^3}{12} - 2\left(\frac{\pi \times 38^4}{64}\right)$ $I = 1955292.3 \text{ mm}^4$ Stress at point A $\sigma_A = \frac{Mc}{I}$ $M = 3 \text{ kN-m} = 3 \text{ x} 10^6 \text{ N-mm}$ c = 30 mm $\therefore \sigma_A = \frac{3 \times 10^6 \times 30}{1955292.3}$ σ_{A} = -46.02 MPa (compression) Stress at point B $\sigma_{B} = \frac{Mc}{I}$ c = 19 mm $\therefore \sigma_{B} = \frac{30 \times 10^{6} \times 19}{1955292.3}$ $\sigma_{\rm B}$ = 29.15 MPa (tension)

You can check your answers online right after the exam. Just go to the course website at:

مع دعواتنا لكم بالتوفيق والنجاح

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